



Appendix B Factual Report







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Plans		
Plan Reference	Revision	Title
GRO-20291-P01	-	Project Location Plan
GRO-20291-P02	-	Exploratory Hole Location Plan





1.0 INTRODUCTION

1.1 Project Objectives

Groundtech Consulting Limited have been instructed by Waterman I&E to undertake a Ground Investigation on the now disused HMP Holloway Prison in Holloway, London. The investigation was conducted in accordance with BS 5930:2015, BS 10175:2017, BS 8576:2013 and BS 22475.

1.2 Proposed Development

The proposed development is predominantly for residential end use comprising approximately 1,000 residential units together with a proportion of commercial floor space, public open space and community space.

1.3 Limitations

Other conditions may exist on the site that have not been taken into account in this assessment as they are outside the scope of works. Groundtech Consulting are not responsible for these circumstances that are not outlined in the report.

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2.0 SITE SETTING

2.1 Location

The site is located circa 2.3km north east of the London Borough of Islington, as shown on the Project Location Plan *GRO-20291-P01* and is approximately centred on National Grid Reference 530098, 185591.



Access to the site is gained off Parkhurst Road / Camden Road to the south east of the site.

2.2 Site Description

The site is approximately rectangular in shape and covers an area of approximately 4.12 hectares.

Onsite Features

The site comprises the now vacant and disused HMP Holloway prison which is a complex of low-rise buildings of two to five storeys in height which were used as cell blocks and various administration and maintenance buildings. Buildings cover approximately 40% of the site with hardstanding and landscaped green spaces covering circa 35% and 25% respectively.

In addition to the cell blocks and office buildings other buildings include a chapel, swimming pool, education centre, visitors centre, day care centre, healthcare unit, day nursery, boiler house and a works and maintenance unit. Ground level varies significantly across the site, generally rising up to an area of higher ground in the south west of the site with a level difference of approximately 8m across the site area.

Pathways, roadways and yards across the site are surfaced with a combination of asphalt, concrete slabs, and block paving. A large tarmac surfaced car park is present in the north east of the site. The remainder of the site comprises landscaped gardens with mature trees and shrubs, surfaced by grass and flower beds.

Boundaries

The prison is bounded by an 8m high wall on its north western and south western boundary, with palisade fencing on its northern boundary and wooden hoarding along its south western frontage with the A503 Camden Road. The site extends beyond the 8m high wall to include a path and soft landscaping which stretches approximately 5m beyond the wall.





Surroundings

The site is surrounded by following features/land uses:

• North - Residential properties.

• East - Parkhurst Road followed by both commercial and residential properties.

• South - Residential properties and a library.

• West - Residential properties.

Site photographs are presented in Appendix 2.





3.0 SCOPE OF INVESTIGATION AND RATIONALE

3.1 Project Objectives

The aim of the fieldwork was to:

Determine the stratification beneath the site.

Maintain a watching brief for visual and olfactory evidence of contamination.

Obtain samples using methodology in current guidance for contamination testing.

Install monitoring standpipes for gas and groundwater monitoring / sampling.

Undertake soil percolation tests in accordance with BRE 365.

Undertake TRL DCP testing to correlate CBR values.

3.2 Scope of Works

The following scope of works was completed between the dates of 18th January and 16th February 2021.

- Twenty one cable percussive boreholes (BH01 to BH21) drilled using a Dando 3000 and a Pilcon 1500 cable percussive drilling rigs and an Eijkelkamp sonic drilling rig to depths between 20.0m and 40.0m bgl.
- Twelve window sample boreholes (WS01 to WS12) were drilled using a Premier Window sample rig to depths between 2.0m and 4.0m bgl.
- Ten machine excavated trial pits (TP01 to TP10) undertaken using a JCB 3CX mechanical excavator to depths between 0.7m bgl and 3.0m bgl.
- One hand excavated trial pit (TP11) to a depth of 1.0m bgl.
- Three soakaway tests (SA01 to SA03) undertaken at depths between 1.9m and 2.7m bgl.
- Four TRL DCP tests undertaken from depths between 0.3m and 0.5m in existing trial pit positions.
- Concrete Coring.
- Full time UXO supervision.
- Underground Utility clearance including GPR.

The exploratory hole locations are presented on Groundtech Plan *GRO-20291-P02* and the exploratory hole logs are presented in *Appendix 3*. The locations and elevations of the exploratory hole locations were surveyed in on completion of the Ground Investigation and are presented on the exploratory logs.

The results of the TRL DCPs are presented in *Appendix 4* and the results of the soil percolation testing are in *Appendix 5*.

The exploratory holes were agreed with Waterman I&E to establish the stratification beneath the site and target any areas of concern. The exploratory holes were logged by a suitably experienced geo-environmental engineer in general accordance with the following current guidance:

- BS 5930 'Code of Practice for Site Investigations' 2015.
- BS EN 14688-1:2002 'Geotechnical Investigation and Testing Identification and classification of soil'.
- BS EN ISO 14689:2002 'Geotechnical investigation and testing Identification and classification of rock'.





3.3 Soil Sampling

During the intrusive investigation, representative samples were taken at regular intervals, changes of strata and where evidence of contamination existed in accordance with the Waterman I&E Specification. The samples obtained are summarised in the table below:

Soil Sample	Number
Environmental Sample	241
Disturbed Sample	676
Bulk Sample	528
Undisturbed UT100 Sample	245

The samples have been obtained in accordance with current environmental and geotechnical guidance. The sampling plan has been designed to obtain samples from all required strata using the correct methodology. Photographs were taken of all environmental samples in accordance with the Waterman I&E Specification.

Disturbed samples of soil for geo-environmental testing were placed in the correct sampling containers as required by the laboratory in accordance with their MCERTS and UKAS Accreditation. Transportation was arranged in a timely manner and the samples were at the correct temperature.

The sample locations and depths are recorded on the exploratory logs.

3.4 Gas and Groundwater Monitoring/Sampling

Gas and groundwater monitoring installations were constructed in the boreholes as instructed by Waterman I&E. The standpipes consisted of high-density polyethylene (HDPE) pipe - a bentonite seal was placed around the plain pipe and a clean gravel pack was placed around the slotted pipe. A 300mm sand bridge was installed between the bentonite and pea gravel. A summary of the installation construction is presented in the table below:

Location	Depth (m bgl)	Response Zone (m bgl)	Targeted Strata	Reason
BH01E	35.00	31.00 - 35.00	Natural Strata	Groundwater
BH01E	3.00	1.00 – 3.00	Made Ground	Ground Gas
BH02	25.00	21.00 - 25.00	Natural Strata	Groundwater
BH02	3.00	1.00 – 3.00	Made Ground	Ground Gas
BH04	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH05	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH06	8.00	5.00 - 8.00	Natural Strata	Groundwater
BH08	8.00	5.00 – 8.00	Natural Strata	Groundwater
BH09	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH10	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH12	8.00	5.00 - 8.00	Natural Strata	Groundwater
BH14	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH16	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH18	8.00	5.00 – 8.00	Natural Strata	Groundwater
BH19	35.00	31.00 – 35.00	Natural Strata	Groundwater
BH21	35.00	31.00 – 35.00	Natural Strata	Groundwater





WS01	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS02	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS05	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS07	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS08	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS10	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS11	4.00	1.00 – 4.00	Made Ground	Ground Gas
WS12	4.00	1.00 – 4.00	Made Ground	Ground Gas

Permanent gas and flow rate monitoring was carried out using a GFM 436 infrared gas monitor with integral electronic flow analyser. The measurements taken are listed below:

- Oxygen (O_2) , carbon dioxide (CO_2) and methane (CH_4) as the percentage volume in air (%v/v).
- Hydrogen sulphide (H_2S) and carbon monoxide (CO) as the percentage volume in air (%v/v).
- Lower Explosive Limit (%LEL) of methane.
- Atmospheric and borehole pressure, including pressure trend.
- Flow measurements (I/hr).
- Weather and ground surface conditions.

Both peak and steady state conditions were monitored to understand the behaviour of the permanent ground gas, the steady state conditions were recorded by allowing the gas monitor to run for a minimum of 3 minutes.

Interim permanent gas and groundwater monitoring results are presented in Appendix 6.

3.5 Groundwater Sampling

Well development was carried out to ensure no cross contamination from the drilling activities remained in the water column in the well. The method of sampling selected was using a low flow pump.

Where possible a water meter was used to test the pH, temperature and conductivity before sampling until equilibrium conditions were met in accordance with BS 10175 guidelines.

Samples were sent to the UKAS Accredited laboratory the same day sampling was carried out in general accordance with BS 5930:2015 and BS 5667.

3.6 Vapour Survey - Photo Ionisation Detector

Standard sampling protocol and preservation of samples was undertaken as described in the EA guidance on Ground Investigation.

Soil was collected for onsite testing, a plastic bag was half filled with soil allowing a suitably sized headspace. The bag was sealed and stored for at least 20 minutes before being tested for Total Volatile Organic Compounds (TVOCs) using a Photo Ionisation Detector (PID).

Results of the PID readings are presented in the remarks on the exploratory hole logs and outlined in the table below.





Location	Sample depth (m bgl)	PID Result (ppm)
BH02	0.50	10.9
BH02	1.00	7.9
BH02	1.70	6.5
BH03	0.50	1.6
BH09	0.50	0.9
BH09	1.00	0.3
BH12	20.00	0.4
BH12	25.00	0.4
BH14	1.00	0.3
BH14	1.50	0.5
BH18	0.50	0.8
BH18	1.00	1.2
BH18	1.50	11.2
BH19	0.50	0.5
BH19	2.00	1.8
BH19	3.00	1.2
BH21	4.00	1.6
BH21	5.00	0.7
BH21	15.00	0.7
WS01	1.50	0.3
WS01	2.00	3.8
WS03	1.00	0.6
WS03	4.00	0.3
WS10	3.00	0.6
WS11	1.00	0.3
WS11	2.00	4.1
WS11	3.00	8.3
WS12	0.20	14.9
WS12	0.50	1.8
WS12	1.00	18.2
WS12	1.50	1.6
WS12	3.00	29.8
TP06	0.50	1.3
TP08	0.50	1.8
TP09	1.00	3.7
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Any PID readings which were ≤0.2ppm are not included in the above table.

The onsite monitoring was carried out in line CIRIA C6658 to aid targeting samples for VOC laboratory analysis.





4.0 GROUND MODEL

4.1 Made Ground

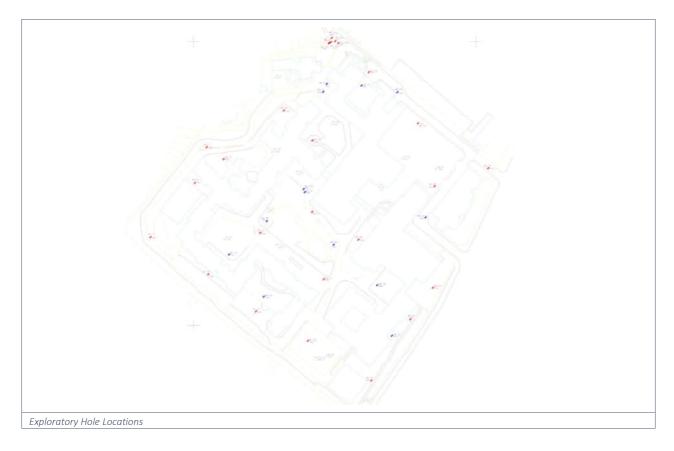
Made Ground was encountered across the entire site to depths of between 0.36m and 2.6m bgl.

The site was surfaced by a mix of concrete, tarmac, block paving and grass over sandy gravelly clay topsoil. The concrete was encountered from the surface to a maximum depth of 0.4m bgl. The topsoil was encountered to a maximum depth of 0.8m bgl.

Three main Made Ground populations were encountered during the investigation and are described below:

- Granular subbase comprising pale brown/orange sandy gravel was generally observed under locations surfaced by tarmac and/or concrete to a maximum depth of 0.6m bgl.
- Relic substructures comprising gravel and cobbles of brick and concrete were encountered in BH01E, BH04, BH07, BH14, TP02, TP03, TP04, TP10, TP11 from 0.3m to a maximum depth of 2.2m bgl.
- Soft brown sandy gravelly clay was the dominant Made Ground strata and was encountered in the majority of locations from depths between 0.1m bgl and 2.6m bgl.

The Made Ground soils were not fully penetrated in TP03, TP10 and TP11 due to excessive thickness of concrete and the proximity of adjacent services.







4.2 Natural Ground

No superficial deposits were encountered during the Ground Investigation.

4.3 Bedrock

Two main bedrock stratification was encountered during the investigation and are described below:

- Soft to firm becoming firm brown slightly gravelly mottled grey Clay from a minimum depth of 0.4m to a maximum depth of 10.7m bgl. This stratum is considered to be oxidised and weathered London Clay.
- Firm to stiff becoming stiff grey Clay with occasional bands of Claystone was encountered from a minimum depth of 8.5m bgl to the base of all the cable percussion boreholes, this stratum is considered to be more competent London Clay.

4.4 Groundwater

No groundwater strikes were observed in the natural Clay soils however some perched water seepage was recorded in the shallow Made Ground soils, these are outlined in the table below:

Location	Depth of strike (m bgl)	Rising to depth (m bgl)
ВН06	0.40	0.40
BH15	3.50	3.00
BH17	1.30	1.30
BH18	1.90	1.90
WS02	0.80	-
TPO2	0.90	-
SA03	0.18	-

The soil percolation test in SA03 was abandoned due to steady ingress of perched water from the granular Made Ground soils.

4.5 Watching Brief

A watching brief was maintained during the Ground Investigation for visual and olfactory evidence of contamination.

Olfactory evidence of contamination was recorded in the locations outlined in the table below:

Location	Depth of odour (m bgl)	Odour description
BH02	0.35 – 1.90	Hydrocarbon
BH03	0.40 – 0.65	Diesel
ВН09	0.20 – 0.60	Organic/Hydrocarbon
BH19	2.00 – 3.20	Hydrocarbon
WS01	1.50 – 2.20	Hydrocarbon
WS12	0.15 – 1.55	Hydrocarbon
TP09	0.40 – 1.35	Hydrocarbon/Diesel





The only visual evidence of potential contamination recorded was ash and rare clinker within the shallow Made Ground soils in a large proportion of the exploratory holes. Additionally, some dark grey/black staining was noted in the Made Ground clay soils in TP09 from depths between 0.8m and 1.35m bgl along the southern face of the trial pit.

4.6 Excavation Stability

All of the trial pit locations were generally stable where deep Made Ground soils were not encountered.

Collapse occurred in WS06 where Made Ground gravel was recorded to 1.75m bgl, this material was too dense to drive casing though and the overlying soils collapsed during drilling trapping the barrel in the hole. Hand excavations were undertaken to try and recover the barrel however they were unsuccessful and the barrel had to be abandoned in the collapsed hole.

4.7 Excavation Progress

Slow progress while excavating the exploratory holes was encountered in multiple locations largely due to thick underlying concrete, relic substructures and Claystone bands. The locations where obstructions/slow drilling were encountered are detailed in the table below:

Location	Depth to obstruction (m bgl)	Thickness of obstruction	Type of obstruction
BH01	Ground level	0.70m+ (Hole terminated at obstruction)	Concrete
BH01A	Ground level	0.75m+ (Hole terminated on obstruction)	Concrete
ВНО1В	Ground level	1.35m+ (Hole terminated on obstruction)	Concrete / Brick substructure
ВН01С	Ground level	0.55m+ (Hole terminated on obstruction)	Concrete
BH01D	Ground level	0.45m+ (Hole terminated on obstruction)	Concrete
BH01E	Ground level	0.80m	Concrete
BH02	11.10m	0.60m	Claystone
ВН06	12.70m	0.30m	Claystone
	18.60m	0.20m	Claystone
	19.20m	0.20m	Claystone
BH10	26.40m	0.30m	Claystone
	28.00m	0.90m	Claystone
BH13	13.10m	0.30m	Claystone
BH18	2.00m	0.45m	Claystone
DUDA	14.90m	0.30m	Claystone
BH21	28.20m	0.20m	Claystone
WS07	0.90m	(Hole terminated at obstruction)	Plastic pipe encountered in inspection pit
TP03	0.80m	0.90m+ (Hole terminated on obstruction)	Brick / Concrete
TDO4	0.97m	0.33m	Concrete
TP04	1.70m	0.50m	Concrete





TP10	0.45m	0.25m+ (Hole terminated on obstruction)	Concrete
TP11	1.00m	(Hole terminated on obstruction)	Concrete





5.0 RELEVANT INDUSTRY REFERENCES

British Standards Institution. Investigation of Potentially Contaminated sites - code of practice. BS 10175:2017.

British Standards Institution 'Code of Practice for Site Investigations' BS 5930:2015

British Standards Institution "Geotechnical investigation and testing – Identification and classification of soil" BS EN ISO 14688:2002.

BS EN ISO 14689:2002 "Geotechnical investigation and testing – Identification and classification of rock".

CIRIA C665 "Assessing Risks Posed by Hazardous Ground Gases to Buildings" 2007.

Wilson & Card "Proposed method classifying gassing sites" Ground Engineering 1999.

Card & Steve Wilson in "A pragmatic approach to ground gas risk assessment for the 21st Century" - CIRIA/Environmental Protection UK Ground gas seminar 2011

BS 8576:2013 'Guidance on investigations for ground gas - Permanent gases and Volatile Organic Compounds (VOCs)'

BS 8485:2015 'Code of practise for the design of protective measures for methane and carbon dioxide ground gases for new buildings'





APPENDIX 1 - Plans



